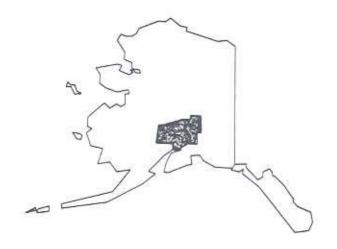


MATANUSKA-SUSITNA
BOROUGH,
ALASKA
MATANUSKA-SUSITNA DIVISION

PRELIMINARY MICHAEL BAKER, JR., INC. MAR 24 1997





### NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for flood plain management and flood insurance purposes. This Flood Insurance Study may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

This publication incorporates revisions to the original Flood Insurance Study. These revisions are presented in Section 9.0.

This preliminary revised Flood Insurance Study contains only profiles added or revised as part of the restudy. These profiles are presented in a reduced scale to minimize reproduction costs. All profiles will be included and printed at full scale in the final published report.

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# PUBLISHED SEPARATELY

Flood Insurance Rate Map Index Flood Insurance Rate Map

### FLOOD INSURANCE STUDY

# 1.0 INTRODUCTION

# 1.1 Purpose of Study

This Flood Insurance Study investigates the existence and severity of flood hazards in the unincorporated areas of Matanuska-Susitna Borough, Alaska, and within the incorporated Cities of Palmer, Houston, and Wasilla, and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study will be used to convert Matanuska-Susitna Borough to the regular program of flood insurance by the Federal Emergency Management Agency. Local and regional planners will use this study in their efforts to promote sound flood plain management.

In some states or communities, flood plain management criteria or regulations may exist that are more restrictive or comprehensive than those on which these federally supported studies are based. These criteria take precedence over the minimum Federal criteria for purposes of regulating development in the flood plain, as set forth in the Code of Federal Regulations at 44 CFR, 60.3. In such cases, however, it shall be understood that the State (or other jurisdictional agency) shall be able to explain these requirements and criteria.

# 1.2 Authority and Acknowledgments

The source of authority for this Flood Insurance Study is the National Flood Insurance Act of 1968, as amended.

The hydrologic and hydraulic analyses for this study were performed by the U.S. Army Corps of Engineers, Alaska District, for the Federal Emergency Management Agency, under Inter-Agency Agreement No. IAA-H-1878, Project Order No. 15. This work, which was completed in April 1982, covered all significant flooding sources affecting Matanuska-Susitna Borough.

A Type 19 Flood Insurance Study was also performed by the study contractor, under Inter-Agency Agreement No. EMW-E-1153, Project Order No. 1, Amendment No. 5. This work was completed in April 1985. It covers portions of Matanuska River and Knik River in the Bodenburg Butte area.

### 1.3 Coordination

A meeting was held on July 20, 1977, and attended by representatives of the study contractor, the Federal Emergency Management Agency, and Matanuska-Susitna Borough. The purpose of the meeting was to determine which streams would require detailed study and which would require approximate study. A priority list of the streams

to be studied was determined; however, due to the lack of funds, the highest priority streams (Matanuska, Susitna, and Knik Rivers) were eliminated from this study. Instead, three streams which have already been partially studied by the U.S. Army Corps of Engineers were funded for this study. The other streams will be studied at a later date when funds are available. Coordination was made with the U.S. Geological Survey and the U.S. Soil Conservation Service. A draft of this study was reviewed by Matanuska-Susitna Borough.

Three final coordination meetings were held during the week of September 29, 1983. These meetings were attended by representatives of the Federal Emergency Management Agency, the study contractor, and the community. All problems and questions that were raised at the meeting have been resolved.

# 2.0 AREA STUDIED

# 2.1 Scope of Study

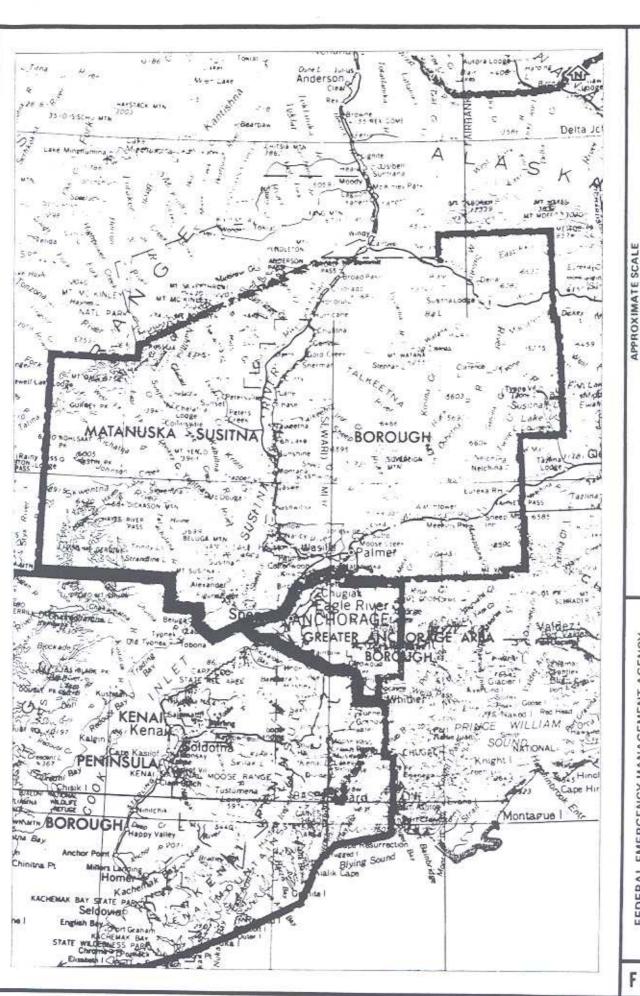
This Flood Insurance Study covers the incorporated Cities of Wasilla, Houston, and Palmer, and the unincorporated areas of Matanuska-Susitna Borough, Alaska. The area of study is shown on the Vicinity Map (Figure 1).

Deception Creek; Deception Creek Tributaries 1, 2, and 3; Willow Creek; Willow Creek Tributary; Little Susitna River; and Little Susitna River Split Flows 1, 2, and 3 were studied by detailed methods. Each stream was studied from a downstream location, below which little development is expected by the borough due to wetland conditions, to an upstream location where the 100-year flood plain is less than 200 feet wide.

Flooding in the Bodenburg Butte area along Matanuska River and Knik River was initially studied by approximate methods. Results from the Type 19 Flood Insurance Study were used to revise the approximate flood boundaries. The latter study evaluated flood hazards on Matanuska River in the vicinity of a flood protection dike along Old Glenn Highway, and on Knik River from Windsong and Heritage Park Subdivision development to Old Glenn Highway Bridge. Information generated by this study is not sufficient to define detail flood boundaries on these streams.

Those areas studied by detailed methods were chosen with consideration given to all proposed construction and forecasted development through 1987.

Approximate analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to and agreed upon by the Federal Emergency Management Agency and the borough.



MAP VICINITY

20 MILES

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FEDERAL EMERGENCY MANAGEMENT AGENCY

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

FIGURE

# 2.2 Community Description

Matanuska-Susitna Borough, located in the Third Judicial District in south-central Alaska, governs an area of over 23,000 square miles extending from the Municipality of Anchorage in the south to Mt. McKinley National Park in the north. It is surrounded by unorganized area to the north, east, and west, and by Greater Anchorage Area and Kenai Peninsula Boroughs to the south. The population of Matanuska-Susitna Borough was 6,509 in 1970 and increased 174 percent to 17,816 by 1980 (Reference 1). It is estimated that over 2,000 people live near the streams studied by detailed methods.

Most of the land along Deception Creek, Willow Creek, and the Little Susitna River is developing as a low-residential area. The water-shed basins for these streams are located in south-central Alaska, approximately 30 air miles and 70 miles by highway north of Anchorage. The area has been a focal point of increasing use for recreational activities such as boating, hiking, snowmobiling, fishing, and hunting. This increased recreational usage can be attributed to the area's esthetic qualities and closeness to Anchorage, the largest city in the State. Tremendous subdivision activities stressing recreational lots have been occurring in recent years.

The streams studied by detailed methods originate in the Talkeetna Mountains and flow west to the Susitna River or Cook Inlet. Physiographic characteristics are quite varied having developed from glacial activities and volcanic action. The study area is underlain primarily by bedrock consisting of weakly consolidated, coal-bearing rocks of Tertiary Age. It has been glaciated several times, so there are thick deposits of glacial drift and alluvial sediments made up of sandy and gravelly material. The relative proportions of the materials in the glacial drift vary quite a bit, as does the compactness of the soil. Thus, permeability and internal drainage are highly variable, even over short distances. Poorly drained soils often occur on the slopes of moraines in close association with well-drained soils. Most of the area is also covered with a mantle of silty loess probably derived from the Susitna River flood plains to the west. The loess ranges from a few inches to several feet in thickness. Poorly drained peat is common in scattered depressions, shallow basins between moraine hills, and other low-lying areas. The Willow area contains fifteen varying vegetative habitat types composed of mature stands of mixed coniferous and deciduous forests with an understory of a variety of forbs and woody plants, muskeg-black spruce bogs, and grassland areas. Ferns, horsetails, and clubmosses are present throughout the entire borough. Generally, the vegetative ground cover is dense and provides substantial protection from erosion activity, particularly in the higher elevations where better drained soil conditions are found. Willow and birch/aspen stands under 10 feet high are generally lacking, with low woody shrubs such as Vaccinum being very common throughout the borough. Elevations range from

10,000 feet in the mountains to less than 100 feet in the southern valleys.

The region is in a transitional climatic zone between maritime and continental conditions. Pronounced temperature variations and cloudy weather are common during a large portion of the year. Mountain ranges to the south act as a barrier to the influx of warm air from the Gulf of Alaska, resulting in an average annual precipitation which is only 10 to 15 percent of that at stations located on the Gulf of Alaska.

Annual precipitation in the study area averages 25 inches with annual snowfall of 80 inches. Rainfall is generally heaviest in August and September with monthly precipitation amounts approximately equal for the rest of the year. The Alaska Mountain range lies in a long arc, approximately 70 miles north of the detailed study area and serves as an effective barrier to the flow of extreme cold winter weather from the north. The streams remain frozen during the winter with ice jam flooding occurring occasionally in the spring. Annual temperatures range from -20°F to 80°F.

# 2.3 Principal Flood Problems

Floods in Matanuska-Susitna Borough can occur as a result of a combination of factors, including heavy snow pack, temperature, sunshine, and precipitation. The sequence of events affects the flooding potential. Spring floods on streams may occur as a result of an above-normal snowfall during the winter followed by an unusually cold spring and a rapid snowmelt. Summer and fall floods usually result from intense precipitation. In addition, an ice jam could occur during the winter or during spring breakup causing overbank flooding. Ice jams have caused the highest flooding on these streams, but no frequency has been applied to this type of flood. Typical of most of Alaska, there is little information available concerning historical floods in Matanuska-Susitna Borough. Public agencies and longtime residents, however, substantiate that floods have occurred. Information of historical floods was obtained primarily from interviews with residents in the area. A tabulation of floods in recent years and an analysis of conditions resulting from these floods are shown in Table 1. The principal flood problems are natural obstructions such as trees and vegetation along the banks, manmade obstructions such as bridges and boatdocks, ice jams, the accumulation of brush and debris along and within the streambed which can be carried downstream by high water and block bridge openings or other constrictions, and inadequately-sized culverts.

Willow Creek crosses the Parks Highway at mile 72. It originates in the Talkeetna Mountains and generally flows west to join the Susitna River. It has a total length of approximately 35 miles of which only the lower 18 is developable. The two major tributaries to this stream are Peters Creek and Deception Creek. The lower reaches of the stream, especially above the Parks Highway, are

# Table 1. Historical Flooding

Year	Flooding Source and Resulting Damage
1938	Willow Creek; water overtopped the railroad, caused by ice jam.
1943	Little Susitna River; pier in railroad bridge washed out.
1949	Little Susitna River; rain on rapid snowmelt caused roads to wash out, damaged culvert.
1955	Willow Creek; heavy rainfall damaged railroad.
1959	Little Susitna River; massive road washouts at Houston and Little Susitna Inn, track and culverts washed out.
1963	Little Susitna River; roads washed out, damaged culverts.
1964	Little Susitna River; ice jam flooding.
1964	Willow Creek; ice jam flooding.
1971	Willow Creek; log jam caused flooding near Willow, damage to highways and residences.
1971	Little Susitna River; railroad undermined at Houston caused derailment of 13 cars. Man-made dam broke during rainfall. Lower Hatcher Pass Road bridge over the Little Susitna River washed out.
1971	Matanuska River; flooding resulted when a landslide-formed dam on Granite Creek (a tributary to the Matanuska River) broke during a period of rainfall and snowmelt. Water overtopped Old Palmer Highway in the Bodenburg Butte area, and residential and commercial buildings were flooded. Discharge was estimated at 80,000 cubic feet per second (cfs). Estimated 100-year discharge for the Matanuska River at Palmer is 40,000 cfs.
1975	Willow Creek; ice and log jams caused flooding. Approximately five homes were flooded off Hatcher Pass Road, 2 to 5 miles east of the Parks Highway.

under intense pressure for subdivision and development in spite of the fact that there are obvious flood hazards within the area.

Deception Creek also originates in the Talkeetna Mountains and generally flows north and west for approximately 20 miles to join Willow Creek just upstream of the Parks Highway. At the present time, the entire length of Deception Creek is sparsely developed with very few crossings.

The Little Susitna River drains the southern slopes of the Talkeetna. Mountains and has its headwaters in the mountains. The land form is such that the river intercepts numerous minor tributaries directly from the mountain slopes to the north. It is an extreme meandering stream and has a total length of approximately 75 miles.

### 2.4 Flood Protection Measures

A dike was constructed along Old Glenn Highway in Bodenburg Butte to prevent spring runoff of Matanuska River from overtopping the highway. The dike does not provide protection against the 100-year flood.

Matanuska-Susitna Borough recently passed a zoning ordinance to restrict development in areas noted for flood hazard. These areas have been determined by previous U.S. Army Corps of Engineers or U.S. Geological Survey studies.

# 3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the borough standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude which are expected to be equalled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for flood plain management and for flood insurance premium rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10, 2, 1, and 0.2 percent chance, respectively, of being equalled or exceeded during any year. Although the recurrence interval represents the long term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood which equals or exceeds the 100-year flood (1 percent chance of annual occurrence) in any 50-year period is approximately 40 percent (4 in 10), and, for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported here reflect flooding potentials based on conditions existing in the borough at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

# 3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for floods of the selected recurrence intervals for each flooding source studied in detail affecting the borough.

Peak discharges for selected recurrence intervals on Deception Creek; Deception Creek Tributaries 1, 2, and 3; Willow Creek; and Willow Creek Tributary were determined utilizing Clarks time-area unit hydrograph analysis sub-routine in the computer program HEC-1-developed by the U.S. Army Corps of Engineers (Reference 2).

Precipitation was determined from the U.S. Weather Bureau Technical Paper No. 53 (Reference 3) and used in the HEC-1 program. These frequencies were confirmed through a regional-frequency analysis developed for other gaged basins in the same geographic area.

Peak discharges for selected recurrence intervals on the Little Susitna River were determined utilizing a regional analysis of drainage area-peak discharge relationships for other stream-gaging stations within the geographic area of the Little Susitna River.

Peak discharge-drainage area relationships for streams studied by detailed methods are shown in Table 2.

The hydrologic analysis included a review of all existing flood frequency data for the area and the utilization of analytical techniques best suited to the specific stream data. Statistical analyses were conducted in accordance with approved procedures recommended by the U.S. Water Resources Council guidelines for determining flood flow frequency (Reference 4).

Reference 19 provides the 100-year peak discharges. It is based on frequency analysis of the data obtained at U.S. Geological Survey gaging station 15,248,000 on Matanuska River. Four recorded discharges resulting from Lake George breakout events (Reference 20) were used for the Knik River analysis.

# 3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of the flooding sources studied in the borough were carried out to provide estimates of the elevations of floods of the selected recurrence intervals along each of these flooding sources.

Water-surface profiles were computed for Willow Creek, Deception Creek, and the Little Susitna River utilizing a computerized HEC-2 step-backwater program developed by the U.S. Army Corps of Engineers (Reference 5).

Cross sections were developed from survey notes, field reconnaissance, photographs, previous studies, and the use of topographic maps at a scale of 1:4,800 (References 6 and 7). Intermediate cross sections were developed utilizing cross-sectional area of

Table 2. Summary of Discharges

Elooding Source and Location	Drainage Area (Square Miles)	Peak D	1scharges 50-Year	Peak Discharges (Gubic Feet per Second) -Year 50-Year 100-Year 500-Yea	Second) 500-Year
Deception Creek At mouth	88	3,650	5,400	6,300	9,000
Deception Creek Tributary 1 At mouth		1,110	1,620	1,840	2,450
Deception Creek Tributary 2 At mouth		1,050	1,550	1,840	2,580
Deception Greek Tributary 3 At mouth	î	069	1,030	1,200	1,720
Willow Creek Downstream of Parks Highway	256	9,800	14,600	16,900	24,200
Willow Creek Tributary At mouth	17-	2,800	4,600	5,900	9,200
Little Susitna River Downstream of Alaska Railroad At Schrock Road Downstream of Welsh Road	169 140 99	8,300 7,400 5,800	12,900 11,400 8,900	15,200 13,500 10,500	21,600 19,000 14,900
Talkeetna River Overflow		2	7	7,000	2
Data not applicable					

the stream in conjunction with the topographic maps. The cross sections were located at close intervals in the vicinity of structures to determine the backwater effects of these structures. In addition, numerous intermediate cross sections have been added when the slope of the stream or the total loss was excessive between any two cross sections. Except where noted in the computations, road and bridge failure was not considered in this study. In effect, the backwaters were computed so as to show the maximum flooding effect regardless of the structure being present.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway is computed (Section 4.2), selected cross section locations are also shown on the Flood Boundary and Floodway Map (Exhibit 2).

Channel roughness factors (Manning's "n") for these computations were assigned on the basis of field inspection of the flood plain areas and standard published factors for utilization of n values through pipes or culverts. The Manning's "n" values used are as follows:

Stream	Roughness Channel	Factors Overbank
Deception Creek		
Deception Creek Tributary 1		
Deception Creek Tributary 2		
Deception Creek Tributary 3		
Willow Creek	0.030-0.035	0.035-0.075
Willow Creek Tributary	0.035	0.035-0.075
Little Susitna River	0.035	0.080-0.120
Little Susitna River		
Split Flow 1	0.035	0.080
Little Susitna River		2007.00
Split Flow 2	0.035	0.120
Little Susitna River	1505(70707)	3,12,00
Split Flow 3	0.035	0.120

Starting water-surface elevations were based on utilization of the slope-area method.

During the early stages of examination of both Willow Creek and the Little Susitna River, it was determined there were several possibilities of divided flow, in essence, split flow throughout the stream reach. It was, therefore, necessary to split these areas, computing one side as a tributary and balancing the flows between the two. In some cases, as will be noted in the computations and on the work maps, floodways were necessary for both portions of this split flow condition.

The Little Susitna River has extensive flow divisions, and three extremely braided areas required split flow analysis. The secondary channels (braids) were modeled as tributaries with water surfaces balancing at the upstream division point.

Approximate 100-year flooding from the Matanuska River along Old Palmer Highway in the Bodenburg Butte area was studied using highwater marks from a flood which occurred in August 1971. Highwater marks were determined from field surveys and interviews with local residents. No recurrence interval was assigned to this event.

Field-surveyed cross sections were used in the Type 19 analysis for Matanuska River and Knik River (Reference 21). Additional cross sectional data were obtained from the Alaska Department of Transportation and Public Facilities. HEC-2 analysis for Matanuska River utilized the levee option to evaluate the effect of the flood protection dike along Old Palmer Highway.

Flood profiles were drawn showing computed water-surface elevations to an accuracy of 0.5 foot for floods of the selected recurrence intervals (Exhibit 1).

All elevations are referenced to the National Geodetic Vertical Datum of 1929 (NGVD). Elevation reference marks used in the study are shown on the maps.

# 4.0 FLOOD PLAIN MANAGEMENT APPLICATIONS

The National Flood Insurance Program encourages State and local governments to adopt sound flood plain management programs. Therefore, each Flood Insurance Study includes a flood boundary map designed to assist communities in developing sound flood plain management measures.

# 4.1 Flood Boundaries

In order to provide a national standard without regional discrimination, the 100-year flood has been adopted by the Federal Emergency Management Agency as the base flood for purposes of flood plain management measures. The 500-year flood is employed to indicate additional areas of flood risk in the community. For each stream studied in detail, the boundaries of the 100- and 500-year floods have been delineated using the flood elevations determined at each cross section; between cross sections, the boundaries were interpolated using topographic maps at a scale of 1:4,800, with a contour interval of 5 feet (References 6 and 7).

Approximate flood boundaries for flooding from the Matanuska River along Old Palmer Highway in the Bodenburg Butte area were initially determined using high-water marks from the 1971 event, and were revised using the Type 19 study results. Delineations were done on topographic maps at a scale of 1:2,400, with a contour interval of 2 feet (Reference 8).

Approximate flood boundaries on Susitna River, Kroto Creek, and Kroto Slough (in the vicinity of their confluences) were delineated based on information supplied by the community.

Approximate flood boundaries in some portions of the study area were taken from the Flood Hazard Boundary Map and from Flood Hazard Studies (References 9 through 16).

Flood boundaries for the 100- and 500-year floods are shown on the Flood Boundary and Floodway Map (Exhibit 2). In cases where the 100- and 500-year flood boundaries are close together, only the 100-year flood boundary has been shown. Small areas within the flood boundaries may lie above the flood elevations and, therefore, not be subject to flooding; owing to limitations of the map scale, such areas are not shown.

# 4.2 Floodways

Encroachment on flood plains, such as artificial fill, reduces the flood-carrying capacity, increases the flood heights of streams, and increases flood hazards in areas beyond the encroachment itself. One aspect of flood plain management involves balancing the economic gain from flood plain development against the resulting increase in flood hazard. For purposes of the National Flood Insurance Program, the concept of a floodway is used as a tool to assist local communities in this aspect of flood plain management. Under this concept, the area of the 100-year flood is divided into a floodway and a floodway fringe. The floodway is the channel of a stream plus any adjacent flood plain areas that must be kept free of encroachment in order that the 100-year flood may be carried without substantial increases in flood heights. Minimum standards of the Federal Emergency Management Agency limit such increases in flood heights to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this report are presented to local agencies as minimum standards that can be adopted or that can be used as a basis for additional studies.

These floodways were computed on the basis of equal conveyance reduction from each side of the flood plain. The results of these computations were tabulated at selected cross sections for each stream segment for which a floodway was computed (Table 3).

Weir flow can occur only to the north of the Alaska Railroad bridge, and a floodway must be left clear along the railroad embankment between Willow Creek and Willow Creek Tributary to permit flow from the main channel to the weir area.

On the Little Susitna River in two of the split flow areas, a split floodway was designed, and in some other areas the floodway is to follow the natural 100-year boundary. The latter was necessary because of excessive velocities and the large number of high ground areas. The extreme meandering nature of this stream required that

FLOODWAY	
WIDTH (FEET)	WIDTH (FEET)
200	200
200	200
200	200
350	350
290	290
350	350
243	243
210	210
300	300
450	450
450	450
450	450
632	632
550	550
650	650
800	800
800	800
400	400
400	400
200	200
285	285
250	250
52	52
439	439
132	132
200	200

'Feet Above Confluence With Willow Creek

FEDERAL EMERGENCY MANAGEMENT AGENCY

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

FLOODWAY DATA

DECEPTION CREEK

TABLE 3

Creek  Creek  Creek  31,230 32,950 379 32,950 37,185 37,185 37,185 37,705 38,340 41,350 295 41,272 31,271 2.9 41,350 42,255 45,0 41,350 42,255 45,0 41,350 42,255 45,0 41,350 42,255 45,0 41,350 41,30				TAVODINAT		3	WATER SURFAC	SURFACE ELEVATION	z
31,230 920 1,296 3.0 282.5 32,950 379 726 4.8 288.3 33,400 162 303 7.4 289.1 34,930 544 772 2.9 296.4 36,460 503 633 3.5 301.9 37,185 579 1,272 3.2 306.0 37,705 300 664 5.6 310.3 38,340 500 1,271 2.9 312.3 44,255 450 664 5.6 310.3 44,255 450 677 6.8 357.4 45,235 324 658 6.4 369.9 46,190 98 402 10.5 378.0 47,400 400 753 5.6 391.3 48,525 184 473 9.0 406.8 49,700 154 468 9.1 418.0 50,750 144 414 10.3 431.2 52,470 259 656 6.5 452.2 53,020 188 523 7.7 464.5	SECTION	The second secon	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	WELOCITY (FEET PER SECOND)		WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
31,230     920     1,296     3.0     282.5       32,950     379     726     4.8     288.3       33,400     162     303     7.4     289.1       34,930     544     772     2.9     296.4       36,460     503     633     3.5     301.9       37,185     579     1,272     3.2     306.0       37,705     300     664     5.6     310.3       41,350     295     596     5.0     312.3       41,350     295     596     5.0     330.7       41,350     295     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     410.3     5.6     452.2       51,705     90     365     6.5     452.2       53,410     259     656     6.5     452.1       52,470     259     656									
32,950 379 726 4.8 288.3 33,400 162 303 7.4 289.1 34,930 544 772 2.9 296.4 36,460 503 633 3.5 301.9 37,185 579 1,272 3.2 306.0 37,705 300 1,272 3.2 306.0 41,350 295 5.0 310.3 44,255 45,160 331 574 5.2 346.1 44,255 45,190 98 402 10.5 378.0 406.8 47,400 400 753 5.6 391.3 44,105 90 365 6.5 6.5 452.2 53,020 188 523 7.7 464.5 5.3 410 188 523 7.7 464.5	AA	31,230	920	1,296	3.0	282 5	282 5	0	•
33,400     162     303     7.4     289.1       34,930     544     772     2.9     296.4       36,460     503     633     3.5     301.9       37,705     300     1,272     3.2     306.0       37,705     300     1,272     3.2     306.0       38,340     500     1,271     2.9     310.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     6.5     6.5     452.2       53,410     188     523     7.7     459.1       52     46.5     7.7     464.5	AC	32,950	379	726		288.3	288 3	200.00	0.3
34,930     544     772     2.9     296.4       36,460     503     633     3.5     301.9       37,185     579     1,272     3.2     306.0       37,705     300     664     5.6     310.3       37,705     30     1,272     3.2     306.0       38,340     500     1,271     2.9     312.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       48,525     184     473     9.0     406.8       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     6.5       53,410     188     521     7.7     459.1       52     7.7     464.5       52     7.7     464.5       7	AD	33,400	162	303	7.4	289 1	280.3	200.0	n .
36,460     503     633     3.5     301.9       37,185     579     1,272     3.2     306.0       37,705     300     1,272     3.2     306.0       37,705     300     1,271     2.9     310.3       41,350     295     596     5.0     312.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       46,190     98     402     10.5     378.0       48,525     184     473     9.0     406.8       48,525     184     473     9.0     406.8       48,525     184     473     9.0     406.8       50,750     144     414     10.3     431.2       51,705     90     365     6.5     6.5     452.2       53,410     188     523     7.7     459.1       53,410     188     523     7.7     464.5       52,470     188     523     7.7     464.5       52,470     464.5     7.7     464.5	AE	34,930	544	772	2.9	296.4			0 0
37,185     579     1,272     3.2     306.0       37,705     300     664     5.6     310.3       38,340     500     1,271     2.9     312.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     6.5     6.5     452.2       53,470     259     656     6.5     452.2       53,410     188     523     7.7     464.5       52,470     188     521     7.7     464.5       52,470     466.5     7.7     464.5       52,470     466.5     7.7     464.5       400     466.5     466.5 </td <td></td> <td>36,460</td> <td>503</td> <td>633</td> <td>3.5</td> <td>301.9</td> <td>301.9</td> <td></td> <td></td>		36,460	503	633	3.5	301.9	301.9		
37,705     300     664     5.6     310.3       38,340     500     1,271     2.9     312.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     455.2       53,410     188     521     7.7     464.5       53,410     188     521     7.7     464.5		37,185	579	1,272	3.2	306.0	306.0	306.9	1.0
38,340     500     1,271     2.9     312.3       41,350     295     596     5.0     330.7       43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     6.5     452.2       52,470     259     656     6.5     452.2       53,410     188     521     7.7     464.5       53,410     188     521     7.7     464.5		37,705	300	664		310.3	310 3	2000	0.0
41,350       295       596       5.0       330.7         43,160       331       574       5.2       346.1         44,255       450       677       6.8       357.4         45,235       324       658       6.4       369.9         46,190       98       402       10.5       378.0         47,400       400       753       5.6       391.3         48,525       184       473       9.0       406.8         49,700       154       468       9.1       418.0         50,750       144       414       10.3       431.2         51,705       90       365       6.5       452.2         52,470       259       656       6.5       452.2         53,410       188       523       7.7       464.5		38,340	200	1,271	1	312.3	312.3	213.6	0.0
43,160     331     574     5.2     346.1       44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     521     7.7     464.5       53,410     188     521     7.7     464.5		41,350	295	596		330.7	330.7	330 9	200
44,255     450     677     6.8     357.4       45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		43,160	331	574	5.2	346.1	346 1	346.1	N 0
45,235     324     658     6.4     369.9       46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		44,255	450	677	6.8	357.4	357.4	357 A	
46,190     98     402     10.5     378.0       47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		45,235	324	658	6.4	369.9	369.9	A 075	
47,400     400     753     5.6     391.3       48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		46,190	98	402	10.5	378 0	278.0		0.0
48,525     184     473     9.0     406.8       49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		47,400	400	753	5.6	391 3	301 3	2.010	7.0
49,700     154     468     9.1     418.0       50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		48,525	184	473	0	406 9	406.0	037.0	0.0
50,750     144     414     10.3     431.2       51,705     90     365     11.6     444.1       52,470     259     656     6.5     452.2       53,020     188     523     7.7     459.1       53,410     188     521     7.7     464.5		49,700	154	468		410.0	0.00	406.8	0.0
52,470 259 656 6.5 452.2 53,020 188 523 7.7 459.1 53,410 188 521 7.7 464.5		50,750	144	414	10.3	431 2	410.0	418.7	0.7
52,470 259 656 6.5 452.2 452 53,020 188 523 7.7 459.1 459 53,410 188 521 7.7 464.5 464		51,705	06	365	11.6	444 1	444 1	431.2	0.0
53,020 188 523 7.7 459.1 459 53,410 188 521 7.7 464.5 464		52,470	259	656	2 4	447.1	1444	T . 6 5 5	0.0
53,410 188 521 7.7 464.5 464		53,020	188	503		7.77	7.764	452.3	0.1
52 22 22 22 72 72 464.5 464		53 410	100	3 5	: 1	T. 60%	459.I	459.1	0.0
		63 000	000	170	··/	464.5	464.5	464.5	0.0
008,66		000,00	300	652	6.1	471.1	471.1	471.2	0.1
54,620 300		54,620	300	099	6.1	479.6	479.6	479.9	0.3
A2 55,530 300 680 5.8 485.7 485.7		55,530	300	680		485.7	485.7	485.7	0.0

Feet Above Confluence With Willow Creek

FEDERAL EMERGENCY MANAGEMENT AGENCY

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

FLOODWAY DATA

DECEPTION CREEK

TABLE 3

FLOODING SOURCE	JACE		FLOODWAY		3	WATER SURFACE ELEVATION	E ELEVATIO	z
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
Deception Creek								
BA BA	55,700,	134	434	9.2	486.8	486.8	486 9	
BB	56,675	490	864	4.6	501.9	501.9	502.5	7.0
BC	58,120,	536	815	4.9	518.0	518.0	519.0	
BD	59,410	142	445	8.9	533.9	533.9	533.9	0.0
BE	59,650,	152	419	8.8	536.3	536.3	536.3	0.0
BF	59,920,	76	350	10.5	539.6	539.6	539.7	0.0
BG	60,610	175	475	7.7	550.5	550.5	550.5	0.0
BH	61,385	267	623	5.9	556.7	556.7	557.1	0.4
BI	61,785	136	410	9.2	561.3	561.3	561.3	0.0
ВЛ	62,410+	135	405	8.7	567.7	2.795	567.7	0.0
Deception Creek								
Tributary 1	•							
А	3802	400	945	1.9	237.6	237.6	238.0	0.4
В	1,0302	400	878	3.4	240.4	240.4	240.5	0.1
υ	1,710,	400	396	2.5	245.5	245.5	245.5	0.0
Д	2,2402	400	859	1.8	247.2	247.2	247.8	9.0
ы	2,7102	400	641	3.3	250.9	250.9	251.0	0.1
Œ4	3,1202	200	1,392	2.0	252.4	252.4	252.9	0.5
9	4,3702	009	1,191	3.5	258.1	258.1	258.3	0.2
н	5,1702	009	1,140	2.3	262.0	262.0	262.2	0.2
Н	6,5202	330	604	3.6	268.9	268.9	269.4	0.5
D	7,6602	300	196	2.7	276.8	276.8	277.3	0.5
M	8,560	200	586	2.4	282 8	28.2 8	283 1	0

<sup>1</sup>Feet Above Confluence With Willow Creek <sup>2</sup>Feet Above Confluence With Deception Creek

FLOODWAY DATA

DECEPTION CREEK-DECEPTION CREEK TRIBUTARY 1

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

z	INCREASE	,	0.4		8.0	9.0	1.0	0.0	2.0	9.0	0.1			0.6	8.0		0.0	
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY NGVD)	1	288.7	296.7	303.4	307.1	311.3	313.8	318.0	330.8	346.3			310.4		6 9	318.8	
BASE F WATER SURFAC	WITHOUT FLOODWAY (FEET		288.3	295.9	302.6	306.5	310.3	313.8	317.7	330.2	346.2			309.8				
M.	REGULATORY	0000	1	295.9	302.6	306.5	310.3	313.8	317.7	330.2	346.2			309.8	312.5	315.4	318.8	
	WELOCITY (FEET PER SECOND)	a	o m	3.0	5.1	2.2	3.8	2.3	3.4	2.9	3.5				3.8	2.7	3.0	
FLOODWAY	SECTION AREA (SQUARE FEET)	469	569	1,014	594	558	289	488	1,145	557	470			231	314	444	305	
	WIDTH (FEET)	185	165	400	253	200	193	490	645	200	300			150	183	200	200	
RCE	DISTANCE	360	680	1,780	2,580	3,000	3,650	3,920	4,620	5,790	7,140			610	1,010	1,770	2,400	
FLOODING SOURCE	CROSS SECTION	Deception Creek Tributary 2 A	8	Ü	Д	ŒJ	Œ	5	Ħ	H	מ	Deception Creek	Tributary 3	A	В	U	a	

Preet Above Confluence With Deception Creek

FLOODWAY DATA

DECEPTION CREEK TRIBUTARY 2-DECEPTION CREEK TRIBUTARY 3

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

	- Chick		F.LOODWAY		3	WATER SURFAC	SURFACE ELEVATION	z
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Willow Creek							(Green	
K	0	1,600	5,438	3.1	107.5	107 5	108 5	
ø.	810	949	2,335	7.2				. · ·
U	3,200	1,505	5,509	3.1		115.4	116.0	0.0
Ω	4,710	1,600	5,357		116.9		117 6	0.0
ы	6,870	650	2,732	6.2	119.8	119.8		
Ŀ	8,870	1,500	6,993	2.4	m	. ~	124.4	
D	12,230	1,700	5,514	3.1	130.3	130.3		
Н	17,780	1,600	5,831	2.9	139.5	139.5	140 5	
I	20,000	1,600	4,315	3.9				
J	22,655	1,920	4,831	3.5			152.0	
M	22,785	2,000	11,745	1.4	155.4	LC.	155.4	
T	23,100	2,100	11,077	1.5			155.5	
Σ	24,590	675	2,163	4.6	156.7	156.7	156.8	
Z	27,215	85	738	13.6	63	163.3	164.2	
0	27,320	1,649	5,355	2.7	167.5	167.5	168.0	
Ъ	33,170	400	1,950	5.9	182.6	182.6		
a	33,870	200	2,762	4.2	184.9	184.9		
H I	34,680	800	2,176	4.6	186.3	186.3		
ו מט	36,630	550	1,768	5.7	197.1	197.1		
H	39,780	793	2,493	4.0	207.5	7	08	7.0
n		373	1,226	8.2	218.4	218.4		
٥	43,700	1,007	2,854	3.5	227.3	227.3	227.5	
34		450	1,583	6.3	248.6	248.6		7.0
×		400	1,694	5.9	250.4			
×	0	800	1,820		261.2	261.2		
2	53,370	811	1,786	5.6	274.8			1 - 12

FLOODWAY DATA

WILLOW CREEK

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

			***************************************		3	WATER SURFAC	SURFACE ELEVATION	z
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	WEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
Willow Creek (Cont'd)								
AA	56,250	180	818	12.2	301.5	301.5	2 105	0
AB	58,550	177	698	14.3	325.4		325.4	
AC	59,070	159	1,073	14.8	331.5	331.5	7.5.5	
AD	62,470	203	1,162	13.7	367.8	367.8	367.8	
AE	66,110	163	1,085		407.8	407.8	407.8	
AF	67,600	190	1,168		427.2	427.2	427.6	
AG	68,430	132	1,006	0.0	439 1	130 1	120.7	
AH	68,470	133	1,009	15.8	452.2		452.2	0.0

FLOODWAY DATA

WILLOW CREEK

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODING SOURCE	UKCE		FLOODWAY		35	WATER SURFAC	SURFACE ELEVATION	z
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	WELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
Willow Creek								
Tributary								
A	1,185	1,450	3,961	1.7	155.7	155.7	156.4	7 0
В	2,665	1,250	5,535	1.2	11.0	0		
U	2,725	931	1,704	4.0	164.0	164.0	164.0	
D	9,700	517	833	3.0	181.2	181.2	181.4	0.2
E	10,920	200	1,313	1.9	185.3	185.3	185.4	0.1
E4	12,355	450	1,102		192.7	192.7	192.7	0.0
D	14,205	009	2,219		198.0	198.0	198.2	0.2
ш	15,650	009	1,255	4.1	204.4	204.4	204.5	0.1
I	17,940	450	1,439	3.6	212.8	212.8		0.5
ט	21,400	650	1,751	3.0	224.5		225.4	0.9
M	23,500	870	1,780	2.9			231.8	0.1
LJ.	26,740	1,197	2,010	2.6	246.5	246.5	246.8	0.3
Σ	28,540	800	1,548	3.3	254.4	254.4	254.8	0.4
Z	29,130	650	1,244	4.2	259.6	259.6	259.8	0.2
0	30,280	200	1,061	4.9	269.5	269.5	269.5	0.0
Д	31,110	250	933	5.6	273.7			0.0
Oř.	31,730	200	1,202	4.3	281.0	281.0	281.1	0.1
×	35,080	009	1,412	3.7	310.4	310.4	310.9	0.5
ß	36,560	177	428	12.1	325.5	325.5	325.7	0.2

Peet Above Confluence With Willow Creek

FLOODWAY DATA

WILLOW CREEK TRIBUTARY

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODWAY DATA

LITTLE SUSITNA RIVER

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

N(	INCREASE			0.3			8.0	9.0			0.3		8.0	0.5	0.4	9.0	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.4	6.0
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY	(GAS)		300.1	304.2	306.6	309.2	311.0	311.1	312.3	319.0	324.8	327.5	331.6	336.8	342.1	346.2	349.0	355.3	357.4	357.8	360.9	363.0	m	m	m	373.1	376.8	380.9
BASE I WATER SURFAC	WITHOUT FLOODWAY	1000		299.8	303.9	305.9	308.4	310.4	310.6	312.0	318.7	324.0	326.7	331.1	336.4	341.5	345.6	349.0	355.3	357.4	357.8	360.9	363.0	363.7	363.7	368.0	372.9	376.4	380.0
3	REGULATORY			299.8	303.9	305.9	308.4	310.4	310.6	312.0	318.7	324.0	326.7	331.1	336.4	341.5	345.6	349.0	355.3	357.4	357.8	360.9	363.0	363.7	363.7	368.0	372.9	376.4	380.0
	VELOCITY (FEET PER SECOND)			2.2	2.8	1.8	2.6	2.0	1.8	4.6	5.0	2.0	2.9	3.3	2.2	3.0	1.7	9.4	6.7	1.3	1.9	2.8	1.5	1.6	2.5	4.5	3.5	5.8	4.9
FLOODWAY	SECTION AREA (SQUARE FEET)			6,329	4,983	7,734	5,045	6,740	7,520	2,864	2,671	6,510	4,666	4,048	5,574	4,052	7,106	1,305	1,837	9,264	6,637	4,329	8,195	7,813	4,863	2,728	3,548	2,113	2,518
	WIDTH (FEET)			1,060	1,060	1,250	1,100	1,200	1,140	875	768	1,337	1,200	1,080	1,150	1,550	1,445	1,765	2,454	3,309	3,003	2,598	2,474	2,482	1,840	1,060	1,010	1,020	630
RCE	DISTANCE			70,780	73,690	75,510	78,600	80,050	80,160	81,183	83,355	86,615	88,840	91,090	94,910	97,145	99,555	101,590	103,145	103,340	103,870	104,610	105,130	105,835	106,050	107,440	109,060	,05	111,985
FLOODING SOURCE	CROSS SECTION	Little Susitna	_	AA	AB	AC	AD	AE	AF	AG	AH	AI	A.J	AK	AL	AM	AN	AO	AP	AQ	AR	AS	AT	AU	AV	AW	AX	AY	AZ

FLOODWAY DATA

LITTLE SUSITNA RIVER

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

FEDERAL EMERGENCY MANAGEMENT AGENCY

TABLE 3

NO	INCREASE				0.0	0.0	0.0	0.0	0.0	0.0	0			0.0	0.0	0.0	0.0	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.3	0.9	1.0	
BASE FLOOD SURFACE ELEVATION	WITH FLOODWAY	Cassu	382 0	286.3	342 2		401.4					418.2	419.2	420.2	423.6	420.0	434.0	437.2	440.2	446.4	447.2	447.4	449.0	451.5		463.7	469.2	
BASE FLOOD WATER SURFACE EL	WITHOUT FLOODWAY		382 8		391.7	396.4	401.2		412.9	413.4	413.7	418.2	419.2	420.2	423.6	429.0	434.0	437.1	440.1	446.4	447.2	447.4	449.0	451.5	457.7	462.8	468.2	475.0
W.	REGULATORY		382.8	386.2	391.7	396.4	401.2	405.9	412.9	413.4	413.7	418.2	419.2	420.2	423.6	429.0	434.0	437.1	440.1	446.4	447.2	447.4	449.0	451.5	457.7	462.8	468.2	475.0
	VELOCITY (FEET PER SECOND)		5.5	8.0	2.8	6.7	2.8	12.5	4.8	6.2	12.4	2.8	5.7	8.0	12.6	10.4	7.8	5.5	10.1	3.6	2.3	3.0	3.5	4.8	4.5	3.3	3.1	3.1
FLOODWAY	SECTION AREA (SQUARE FEET)		2,243	1,536	4,405	1,825	4,048	770	1,984	1,550	772	3,967	979	269	444	535	716	1,021	250	1,559	2,424	1,885	1,603	2,349	2,315			3,378
	WIDTH (FEET)		745	444	066	929	1,949	165	099	222	109	1,732	200	91	96	100	195	160	120	758	840	750	449	970	720	750	1,025	1,135
RCE	1 DISTANCE		113,200	114,385	116,365	118,335	120,085	121,860	123,240	123,470	124,055	124,520	125,095	125,620	126,510	127,190	128,135	129,035	130,495	131,835	132,060	132,280	133,260	134,115	135,645	136,925	138,315	139,685
FLOODING SOURCE		Little Susitna River (Cont'd)	BA	BB	BC	ВД	BE	BF	BG	BH	BI	BJ	BK	BL	BM	BN	BO	BP	BQ	BR	BS	BT	BU	BV	BW	BX	BY	BZ

FLOODWAY DATA

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LITTLE SUSITNA RIVER

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

TECODING SOURCE	URCE		FLOODWAY		3	BASE FLO WATER SURFACE	FLOOD CE ELEVATION	z
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	WEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY (FEET	WITH FLOODWAY NGVD)	INCREASE
Little Susitna River (Cont'd)			8					
CA	140,770	1,260	3,421	3.1	481.0	481.0	482.0	0
CB	141,815	086	3,164	3.3	486.7	486.7	487.6	0.40
ဗ	142,640	1,020	2,500	4.2	492.2	492.2	492.4	0.2
9	143,505	820	2,424	4.3	496.2		496.8	9.0
CE	144,835	620	1,837	5.7	502.5		502.7	0.0
CF	146,170	365	1,478	7.1	509.8		510.0	0.2
8	147,455	1,100	2,691	3.9	516.3	516.3	516.6	
CH	148,590	495	2,241	4.7	519.7	519.7	520.6	0.9
CI	149,985	1,000	2,390	4.4	526.8	526.8	527.1	
CJ		740	2,975	2.9	532.7	532.7	533.5	0.8
CK	_	171	451	9.5	535.7	535.7	535.7	0.0
CL	N	800	1,901	2.3	537.9	537.9	538.5	9.0
ð	152,450	691	1,684	2.6	541.3	541.3		0.4
S	54,	700	1,843	2.3	547.3	547.3	548.2	0.9
00	155,405	350	768	5.6	554.3	554.3	554.5	0.2
CP	156,755	239	615	7.0	560.9	560.9		0.0
87	157,620	200	416	10.3	563.2	563.2	563.2	0.0
CR	158,070	250	475	9.1	570.8	570.8	570.8	0.0
CS	158,955	115	452	9.5	576.6	576.6	576.6	0.0
CI	159,665	200	391	11.0	583.3	583.3	583.3	0.0
9	160,530	2,008	1,395	3.1	587.4	587.4	587.4	0.0
CV	161,430	2,000	1,898	2.3	595.5	595.5	595.5	0.0
3	162,125	2,280	4,516	1.9	597.7	597.7	597.7	0.0
Č	162,980	800	2,160	4.0	602.6	602.6	602.6	0.0
CZ	163,845	1,500	1,846	4.7	609.3	609.3	609.3	0.0
CZ	164,715	2,400	4,455	1.9	617.1	617.1	617.1	0

FLOODWAY DATA

LITTLE SUSITNA RIVER

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

z	INCREASE			c	0.0			0.0										0.0		0.0	0.0	0.0						0.0	0.0
FLOOD CE ELEVATION	FLOODWAY	Toron .		0 663	· u		7 6				1 1	666.0	670.5	674.1		·		6.95.9	V		710.1	718.5	726.3	734 3	736.5		7.057	750.7	
BASE FLA	WITHOUT FLOODWAY			622 0	626 9	632.0	636.1					666.0	670.5		681.0			695.9		703.9	710.1	718.5					746 7	752 0	
×	REGULATORY			622.0				644.8	647.8	651.9	657.7	0.999	670.5	674.1	681.0	687.2	691.7	695.9	9.669	703.9	710.1	718.5	726.3	734.3		746.7		752.9	
	VELOCITY (FEET PER SECOND)			2.7	2.6	2.0	2.0	3.5		3.0	4.9	3.4	4.7	5.2	3.6	3.2	2.4	2.8	2.7	3,3	2.8	4.9	4.5	3.6	4.0	10.7	10.7		
FLOODWAY	SECTION AREA (SQUARE FEET)			3,151	3,265	4,232	4,215	2,176	5,267	2,607	1,557	2,250	1,638	1,475	2,159	2,401	3,147	2,774	2,828		-	1,556	1,720	2,143	1,947	720	720	837	694
	WIDTH (FEET)			1,000	1,040	2,640	1,520	1,600	1,800	800	450	620	200	550	1,250	1,600	1,880	1,250		1,000	820	009	200	906	708	294	294	328	187
RCE	DISTANCE			165,610	166,405	167,230	168,070	168,905	169,565	170,245	171,115	171,980	172,300	172,600	173,225	173,870	174,210	174,565	174,950	175,250	175,815	176,400	176,970	177,770	178,015	178,265	178,695	179,125	179,470
FLOODING SOURCE		Little Susitna	River (Cont'd)	DA	DB	2	QQ	DE	DF	2	НО	DI	D	DK	DE	Æ	NO	8	DP	8 1	A I	S	TA I	DQ	DQ	DW	DX	DY	DZ

FLOODWAY DATA

LITTLE SUSITNA RIVER

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

000	FLOODING SOURCE		FLOODWAY	MEAN	2	MATER SURFACE EL	BASE FLOOD SURFACE ELEVATION	z
	DISTANCE	WIDTH (FEET)	AREA (SQUARE FEET)	VELOCITY (FEET PER SECOND)	REGULATORY	FLOODWAY (FEET	FLOODWAY NGVD)	INCREASE
10								
	200	3,309	9,264	1,3	357.4	357 4	167 4	c
	009	3,003	6,637	1.9				0.0
	1,150	2,598	4,329	2.8	360 9	360 0	0.000	0.0
	1,550	2,474	8,195	1.5		363.0	363.0	
	1,800	2,482	7,813	1.6	363.7	363.7	363.7	0.0
	120	1,949	4,048	2.8	401.2	401.2	401 4	0
	2,220	1,164	1,459	1.2			407.2	
	3,420	515	1,026	1.7	414.0	414.0	414.0	0.0
	3,880	773	1,321	1.3	417.4	417.4	417.4	0.0
	4,070	816	3,955	2.9	418.1	418.1	418.1	0.0
-	4,530	1,731	3,955	2.9	418.1		418.1	
	5,720	450	2,151	2.7	420.2	420.2	420.3	2.0
	7,025	300	1,462	3.9	423.4		424.4	1.0
	8,650	620	2,190	2.6	427.8	427.8	28	1.0
_	9,780	009	1,795	3.2	432.1	432.1	432.5	0.4
	10,560	200	1,119	5.1	436.4	436.4	436.6	
	11,110	009	2,174	2.6	438.4	438.4	438.9	
	11,995	800	1,995	2.9	442.7	442.7	442.7	0.0
	12,490	1,125	5,232	1.1	444.5	444.5	442.7	
_	12,620	1,194	2,405	2.4	447.0			0.0
_		820	2,837	2.0	448.5	448.5	448.5	0.0
-	13,715	670	1,333	4.3	448 9	448 9	440.0	

Peet Above Confluence With Little Susitna River

FLOODWAY DATA

1 ......

LITTLE SUSITNA RIVER-SPLIT FLOW 1-LITTLE SUSITNA RIVER-SPLIT FLOW 2

TABLE 3

FEDERAL EMERGENCY MANAGEMENT AGENCY

z	INCREASE	0.7 0.1 0.0 0.0 0.0 0.0 0.0
LOOD E ELEVATION	WITH FLOODWAY NGVD)	536.3 540.7 542.5 542.5 555.7 567.3 570.6 570.6 580.7 589.6 593.9
BASE FLOOD WATER SURFACE EL	WITHOUT FLOODWAY (FEET	535.6 540.5 542.4 548.4 555.7 566.5 576.5 586.9 586.9 593.9
3	REGULATORY	535.6 540.5 542.4 542.4 555.7 556.5 570.6 576.5 589.6 593.9
	WELOCITY (FEET PER SECOND)	4.2.2.2.2.4.E.E.E.E.E.E.E.E.E.E.E.E.E.E.
FLOODWAY	SECTION AREA (SQUARE FEET)	1,048 1,131 811 1,548 1,177 1,593 1,747 1,680 926 1,184 1,294 1,346
	WIDTH (FEET)	620 330 340 480 425 630 1,020 680 740 491
JRCE	DISTANCE 1	450 1,265 1,380 2,940 4,240 5,775 6,515 7,960 8,610 9,240 9,240
FLOODING SOURCE	CROSS SECTION	Little Susitna River A B C C C C B R C L L

1Feet Above Confluence With Little Susitna River

FLOODWAY DATA

LITTLE SUSITNA RIVER SPLIT FLOW 3

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

FEDERAL EMERGENCY MANAGEMENT AGENCY

TABLE 3

the floodways go from meander to meander rather than attempt to follow the stream.

As shown on the Flood Boundary and Floodway Map (Exhibit 2), the floodway widths were determined at cross sections; between cross sections, the boundaries were interpolated. In cases where the boundaries of the floodway and the 100-year flood are either close together or collinear, only the floodway boundary has been shown.

The area between the floodway and the boundary of the 100-year flood is termed the floodway fringe. The floodway fringe thus encompasses the portion of the flood plain that could be completely obstructed without increasing the water-surface elevation of the 100-year flood more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to flood plain development are shown in Figure 2.

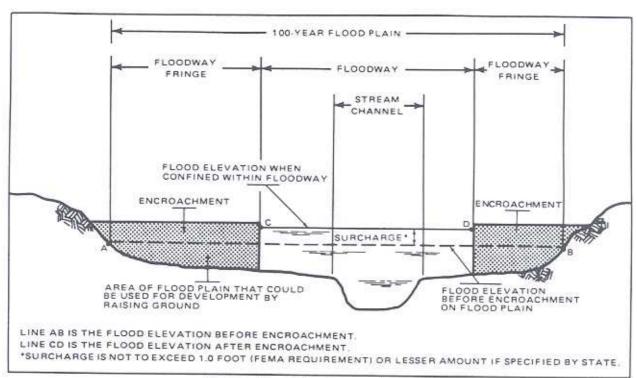


Figure 2. Floodway Schematic

# 5.0 INSURANCE APPLICATION

In order to establish actuarial insurance rates, the Federal Emergency Management Agency has developed a process to transform the data from the engineering study into flood insurance criteria. This process includes the determination of reaches, Flood Hazard Factors, and flood insurance zone designations for each flooding source studied in detail affecting Matanuska-Susitna Borough.

# 5.1 Reach Determinations

Reaches are defined as lengths of watercourses having relatively the same flood hazard, based on the average weighted difference in water-surface elevations between the 10- and 100-year floods. This difference does not have a variation greater than that indicated in the following table for more than 20 percent of the reach:

Average Difference Between 10- and 100-Year Floods	Variation
Less than 2 feet	0.5 foot
2 to 7 feet	1.0 foot
7.1 to 12 feet	2.0 feet
More than 12 feet	3.0 feet

The locations of the reaches determined for the flooding sources of Matanuska-Susitna Borough are shown on the Flood Profiles (Exhibit 1) and summarized in Table 4.

# 5.2 Flood Hazard Factors

The Flood Hazard Factor (FHF) is the Federal Emergency Management Agency device used to correlate flood information with insurance rate tables. Correlations between property damage from floods and their FHF are used to set actuarial insurance premium rate tables based on FHFs from 005 to 200.

The FHF for a reach is the average weighted difference between the 10- and 100-year flood water-surface elevations expressed to the nearest one-half foot, and shown as a three-digit code. For example, if the difference between water-surface elevations of the 10- and 100-year floods is 0.7 foot, the FHF is 005; if the difference is 1.4 feet, the FHF is 015; if the difference is 5.0 feet, the FHF is 050. When the difference between the 10- and 100-year water-surface elevations is greater than 10.0 feet, accuracy for the FHF is to the nearest foot.

# 5.3 Flood Insurance Zones

After the determination of reaches and their respective FHFs, the entire unincorporated area of Matanuska-Susitna Borough was divided into zones, each having a specific flood potential or hazard. Each zone was assigned one of the following flood insurance zone designations:

Zone A:

Special Flood Hazard Areas inundated by the 100-year flood, determined by approximate methods; no base flood elevations shown or FHFs determined.

FLOODING SOURCE	PANEL	BETWEEN 18	EEN 1% (100-YEAR) FLOC	BETWEEN 18 (100-YEAR) FLOOD AND	FLOOD	arrow.	BASE FLOOD
		10% (10-YEAR)	2% (50-YEAR)	2% 0.2% (50-YEAR)	FACTOR	ZONE	ELEVATION 3 (FEET NGVD)
Deception Creek							
Reach 1	7945	-1.0	-0.2	0	010	c	
Reach 2	7945.7965		-0.2		200	A.C.	1 266
Reach 3	7965 9780			0.0	000	AT	see.
2 1000	0000,0000		٠.	`.	070	A2	Varies - See Map
Keach 4	8780,8785	-0.5	-0.2	0.4	900	Al	
Reach 5	8780,8785	-1.7	-0.4	1.0	015	A3	Cop -
Reach 6	8785	6.0-	-0.2	9.0	010	A2	0000
Reach 7	8785	-1.5	-0.3	6.0	015	A3	- See
Deception Creek Tribu-							
tary 1							
Reach 1	7965	-0.5	-0.1	0.3	005	1.0	C
Reach 2	7965 8790			1 0	0 0	1	aac I
	201000	7 - 7 -	5.0-		010	AZ	Varies - See Map
Deception Creek Tribu-							
tary 2							
Reach 1	8780	-1.1	-03	7 0	010	0.5	
Reach 2	8780	-0.4	-0.1	0.3	500	Al	
Deception Creek Tribu-							
tary 3							
Reach 1	8780,8785	-0.4	-0.1	0.3	500	Al	Varies - See Map
Willow Crook							
TOW CLOCK	0 0	1	200		247800000	-	33
Reach 1	7940,7945	-1.07	-0.29	0.78	010	A2	Varies - See Map
Reach 2	7945	-1.71	-0.73	1.00	015	A3	Varies - See Map
Reach 3	7965	-1.10	-0.16	0.62	010	A2	Spp
Reach 4	7965 7970	-2 19	-0 47	1 20	000	* *	1 1

Plood Insurance Rate Map Panel Weighted Average

FEDERAL EMERGENCY MANAGEMENT AGENCY

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

Rounded to Nearest Foot

# FLOOD INSURANCE ZONE DATA

DECEPTION CREEK-DECEPTION CREEK TRIBUTARY 1.

DECEPTION CREEK TRIBUTARY 2-DECEPTION CREEK TRIBUTARY 3-WILLOW CREEK

	ZONE ELEVATION 3 (FEET NGVD)	A3 Varies - See Map A2 Varies - See Map A4 Varies - See Map	
FLOOD	FACTOR	015 010 020	
ENCE FLOOD AND	0.2% (500-YEAR)	1.00	
ELEVATION DIFFERENCE BETWEEN 1% (100-YEAR) FLOOD AND	2% (50-YEAR)	-0.73 -0.16 -0.47	
ELEVATION BETWEEN 1% (10	10% (10-YEAR)	-1.71 -1.10 -2.19	
PANEL.		7945 7965	
FLOODING SOURCE		Willow Creek Tributary Reach 1 Reach 2 Reach 3	

FLOOD INSURANCE ZONE DATA

WILLOW CREEK TRIBUTARY

TABLE 4

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODING SOURCE	PANEL 1	ELEVAT BETWEEN 13	ELEVATION DIFFERENCE EEN 1% (100-YEAR) FLOC	BETWEEN 1% (100-YEAR) FLOOD AND	FLOOD	ZONE	
		(10-YEAR)	(50-YEAR)	0.2% (500-YEAR)	FACTOR		
Little Susitna River							
Reach 1	8795,9610,	-1.46	-0.47	0.92	015	A3	_
Reach 2	8795,8815,				035	A7	_
Reach 3	9610,9630 8815,8820	-1.84	-0.51	1.23	020	A4	_
	8865,9630						-
Reach 4	8865	-1.21	-0.37	0.88	010	A2	_
	8865	-2.21	-0.35	0.91	020	A4	_
Reach 6	8865,8870	-1.50	-0.36	0.72	015	A3	
	8870	-2.65	-0.56	0.95	025	A5	_
Reach 8	8880,8890, 8860,8870	-1.14	-0.37	0.76	010	A2	
Little Susitna River -							
Split Flow 1 Reach 1	8845	-1.99	-0 31	0 0	020	*	then in a
Little Consister Disse			1	3	2	č	
Split Flow 2							
Reach 1	8845,8865	-2.12	-0.60	1.10	020	A4	Varies - See Map
Little Susitna River -							
Split Flow 3							
Reach 1	8865,8870	-1.50	-0.36	0.72	015	A3	Varies
Reach 2	8870	-2.65	-0.56	0.95	025	A5	Varies
Reach 3	8870	-0.85	-0.32	0.91	010	A2	Varies

1 Flood Insurance Rate Map Panel Weighted Average

FEDERAL EMERGENCY MANAGEMENT AGENCY

MATANUSKA-SUSITNA BOROUGH, AK (MATANUSKA-SUSITNA DIVISION)

3 Rounded to Nearest Foot

# FLOOD INSURANCE ZONE DATA

LITTLE SUSITNA RIVER-LITTLE SUSITNA RIVER-SPLIT FLOW 1.
LITTLE SUSITNA RIVER-SPLIT FLOW 2-LITTLE SUSITNA RIVER-SPLIT FLOW 3

TABLE 4

Zones Al through A5 and A7:

Special Flood Hazard Areas inundated by the 100-year flood, determined by detailed methods; base flood elevations shown, and zones subdivided according to FHFs.

Zone B:

Areas between the Special Flood Hazard Areas and the limits of the 500-year flood, including areas of the 500-year flood plain that are protected from the 100-year flood by dike, levee, or other water-control structure; also areas subject to certain types of 100-year shallow flooding where depths are less than 1.0 foot; and areas subject to 100-year flooding from sources with drainage areas less than 1 square mile. Zone B is not subdivided.

Zone C:

Areas of minimal flooding.

Zone D:

Areas of undetermined, but possible flood hazard.

The flood elevation differences, FHFs, flood insurance zones, and base flood elevations for each flooding source studied in detail in the community are summarized in Table 4.

# 5.4 Flood Insurance Rate Map Description

The Flood Insurance Rate Map for Matanuska-Susitna Borough is, for insurance purposes, the principal result of the Flood Insurance Study. This map (published separately) contains the official delineation of flood insurance zones and base flood elevation lines. Base flood elevation lines show the locations of the expected whole-foot water-surface elevations of the base (100-year) flood. This map is developed in accordance with the latest flood insurance map preparation guidelines published by the Federal Emergency Management Agency.

# 6.0 OTHER STUDIES

The U.S. Soil Conservation Service has prepared three Flood Hazard Studies, two Flood Plain Management Studies and a Flood Plain Inventory Report for various streams in Matanuska-Susitna Borough (References 11 through 16). These reports were the sources of some of the approximate flood boundaries presented in this study.

The Expanded Flood Plain Information Study for Willow, Alaska (Reference 17) utilized the same hydrologic and hydraulic procedures; however, through the use of spatial analysis, it reported the effects of flooding and development on the environment and considered the effects of evacuation, floodproofing, and zoning on existing conditions in the year 2000. This study is in agreement with the Flood Plain Information Study.

A Flood Insurance Study has been prepared for the Municipality of Anchorage (Reference 18). This study is in agreement with the Anchorage Flood Insurance Study. Flood Hazard Boundary Maps have been prepared for the unincorporated areas of Matanuska-Susitna Borough and the City of Palmer (References 9 and 10). Due to the more detailed nature of this study, it supersedes the Flood Hazard Boundary Maps.

This study is authoritative for the purposes of the National Flood Insurance Program; data presented herein either supersede or are compatible with all previous determinations.

# 7.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting the Federal Emergency Management Agency, Mitigation Division, Federal Regional Center, 130 228th Street, SW, Bothell, Washington 98021-9796.

# 8.0 BIBLIOGRAPHY AND REFERENCES

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### 9.0 REVISION DESCRIPTIONS

This section has been added to provide information regarding significant revisions made since the original Flood Insurance Study was printed. Future revisions may be made that do not result in the republishing of the Flood Insurance Study report. To assure that any user is aware of all revisions, it is advisable to contact the community repository of flood-hazard data located at Matanuska-Susitna Borough, Code Compliance, 350 East Dahlia Avenue, Palmer, Alaska 99645.

### 9.1 First Revision

This restudy was revised on \_\_\_\_\_\_\_, to show modifications to the flood hazards caused by overflow water from the Talkeetna River within portions of the community of Talkeetna. The Talkeetna River Overflow was modeled along the north embankment of the Alaska Railroad from the east bank of the Talkeetna River to the Talkeetna Spur Highway, approximately 1.4 miles downstream.

The hydrologic and hydraulic analyses for this revision were performed by the U.S. Army Corps of Engineers (USACE), Alaska District, for the Federal Emergency Management Agency, under Interagency Agreement No. EMW-95-E-4759. This work was completed in October 1996.

The results of this revision were reviewed at a final Consultation Coordination Officer meeting held on \_\_\_\_\_\_, and attended by representatives of \_\_\_\_\_\_. All problems raised at that meeting have been addressed in this restudy.

The abutments of the Alaska Railroad bridge over the Talkeetna River constrict the flow of the river causing upstream floodwaters to overflow to the east. This overflow is conveyed between an embankment to the south, which elevates the railroad approach to the bridge, and higher ground to the north. The floodwaters eventually flow over a low section of the railroad embankment and into the Susitna River. The USACE report entitled "Flood Plain Information, Talkeetna River - Susitna River - Chulitna River, Talkeetna, Alaska" (Reference 22) provided a 100-year-flood peak discharge of 97,000 cfs for the Talkeetna River at its mouth. An overflow discharge of 7,000 cfs was calculated through a balancing of energy grade lines immediately upstream of the bridge in hydraulic models of the Talkeetna River and the Talkeetna River Overflow.

Cross sections for the hydraulic model of the Talkeetna River Overflow were field surveyed by the USACE. Additional spot elevations were provided by the USACE for use in interpolating flood plain boundaries between cross sections. Water-surface elevations for the 100-year flood were computed using Version 1.1 of the USACE HEC-RAS computer program (Reference 23). Two 3-foot-diameter culverts are modeled under Talkeetna Spur Highway. These culverts do not convey the entire 7,000 cfs flow; thus, floodwaters weir over the road. Roughness values were chosen based on field observations and ranged from 0.035 to 0.100.

Table 2, "Summary of Discharges," and Exhibit 1, "Flood Profiles," were also revised to reflect changes as a result of the restudy.

